

## **MEMO**

TO: Eric Osborne DATE: 20 July 2025

FROM: Tony Trueman PROJECT J000814

NO.:

COPY: Craig Martell, Susan Jones

SUBJECT: PEER REVIEW RESPONSES TO 100 & 110 TE MOANA ROAD, FLOOD ASSESSMENT &

STORMWATER MANAGEMENT CONCEPT MEMO

#### **INTRODUCTION:**

Te Miro Water have made a further information request regarding the flood assessment & stormwater management concept memo supporting the plan change.

The following points have been raised by Te Miro Water.

- 1. RFI Update information provided around the effects of the Chillingworth stop bank breach.
- 2. RFI Provide detailed soakage test information.
- 3. RFI Provide full Geotechnical Report and method for establishing seasonal high groundwater elevation.
- 4. RFI Provide a clear solution of how the stormwater associated risks are managed for flood plain filling and peak flow rate mitigation.
- 5. RFI Provide a clear solution of how water quality and runoff volume will be managed.

### Response - Point 1:

RFI – Update information provided around the effects of the Chillingworth stop bank breach.

There are impacts on peak depths within the site associated with the Chillingworth stop-bank breach scenario.

- 1. Breach flows move north-west toward the expressway following the lowest topography.
- 2. The low-lying ground adjacent the expressway stores some of the flow moving north.
- 3. Flow moves under the expressway and into the site.
- 4. Flow moves over Te Moana Road and into the Waimeha Stream

Peak depths within the site are in the order of 10 mm to 600 mm in this breach scenario.

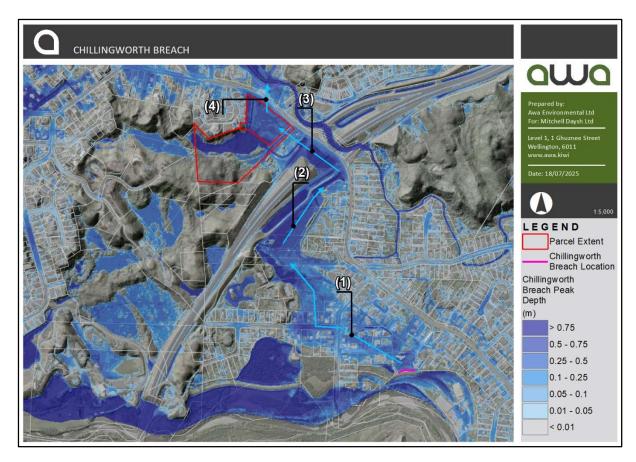


Figure 1 Chillingworth Stop-Bank Breach Scenario Peak Depths

There is sufficient scope, through site earthworks, to maintain the "effective functionally" of the residual overflow path through the site post development. A specific flood hazard assessment will be undertaken, including the development design, as part of future resource consent applications.

#### Response – Point 2:

RFI - Provide detailed soakage test information.

See detailed soakage test information in Appendix 1.

#### Response - Point 3:

RFI – Provide full Geotechnical Report and method for establishing seasonal high groundwater elevation.

The ground water levels have been shown to vary across the site in response to seasonal, wet weather events, soil type and local drainage. Awa and Cuttriss both encountered groundwater at varying depths using a hand auger. Within Lot 1 AWA encountered ground water at approximately 1.5 metres below ground level while Cuttriss encountered ground water at approximately 600mm below ground level. CGW have assumed a ground water 1.0 metre below ground level.

It is recommended by Cuttriss, CGW and AWA that standpipe piezometers are installed across the site along with regular water level monitoring to determine the groundwater level across the site.

#### Response - Point 4:

RFI – Provide a clear solution of how the stormwater associated risks are managed for flood plain filling and peak flow rate mitigation.

This is a flood assessment & stormwater management concept memo for a plan change application. The purpose of the memo is to provide an overview of the local and regional flood hazard associated within the site and whether there is sufficient scope to mitigate development of the site. A specific flood hazard assessment will be undertaken, including the development design, as part of future resource consent applications.

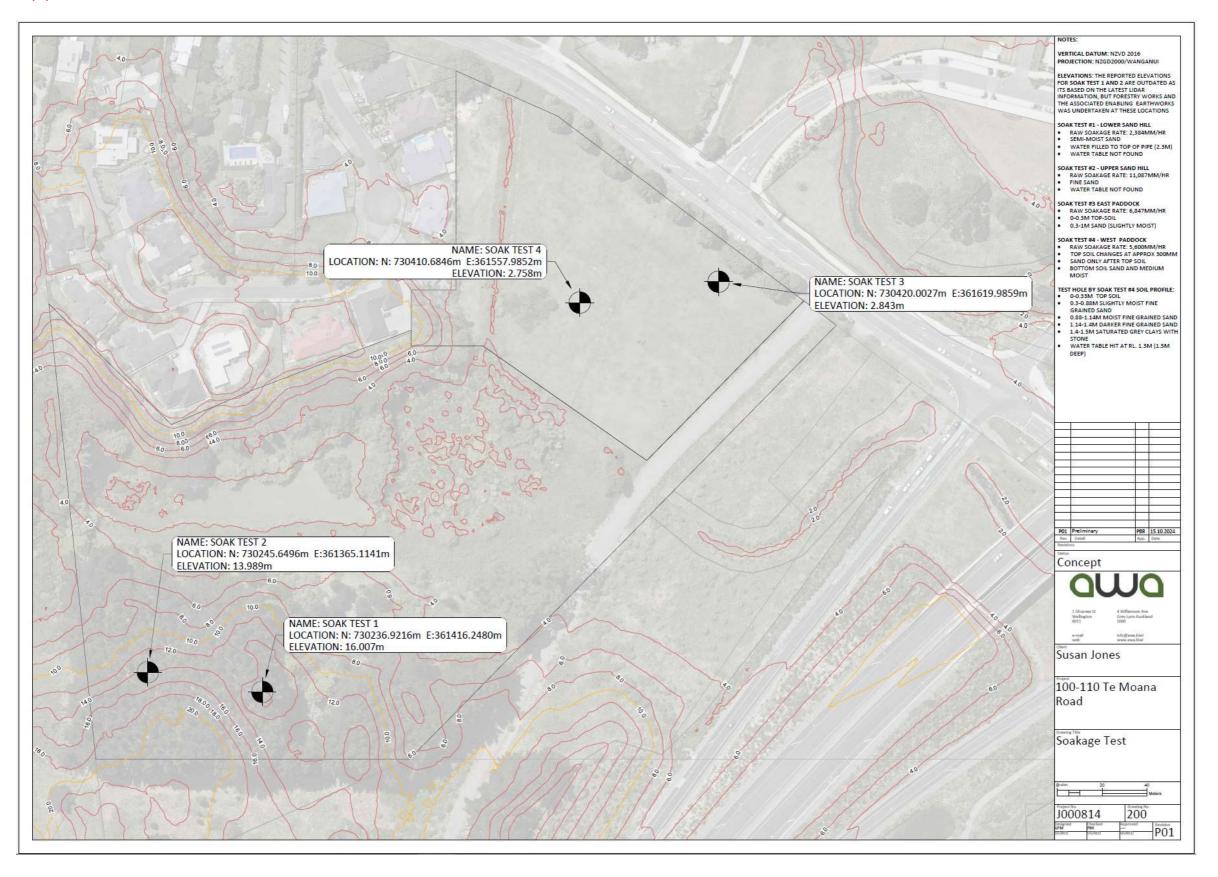
### Response – Point 5:

RFI - Provide a clear solution of how water quality and runoff volume will be managed.

There are several options available to maintain water quality across the site. These could take the form of infiltration or attenuation devices such as soakage/storage crates, bio-infiltration devices or constructed wetland areas. All these devices return runoff to ground in a diffuse manner which will mimic pre-development runoff patterns while maintain water quality.

A specific flood hazard assessment for mitigating runoff volume associated with the development design will be undertaken as part of future resource consent applications.

## Appendix 1:

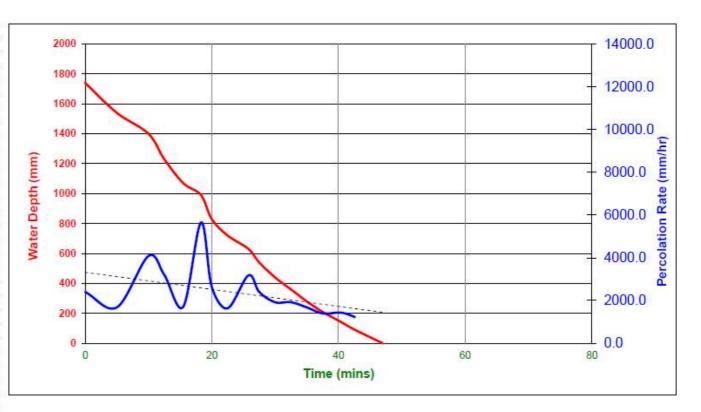


#### Percolation Test Results (NZBC & Side and head wall)



Pipe Depth (m) = 2.34 m Bore DIA (mm) = 80 Soakaged Volume | Percolocation Rate Time Water Level Water Depth Soakage Area (m<sup>3</sup>) (minutes) (mm below surface) (m<sup>2</sup>)(mm/hr) 600 1740 0.0050 1540 0.00101 5 800 0.0050 2400.0 1400 0.00070 1680.0 10 940 0.0050 0.00085 1110 1230 0.0050 12.5 4080.0 0.0050 0.00080 3200.0 15.5 1270 1070 18.3 1350 990 0.0050 0.00040 1714.3 20 1510 830 0.0050 0.00080 5647.1 22.5 720 2640.0 1620 0.0050 0.00055 630 540 1636.4 3176.5 1710 0.0050 0.00045 27.5 1800 0.0050 0.00045 440 2400.0 30 1900 0.0050 0.00050 32.5 1980 360 0.0050 0.00040 1920.0 35 2060 280 0.0050 0.00040 1920.0 37.5 2130 210 0.0050 0.00035 1680.0 1400.0 140 40.5 2200 0.0050 0.00035 1428.6 1227.3 2250 2340 90 0.00025 42.6 0.0050 47 0.0050 0.00045

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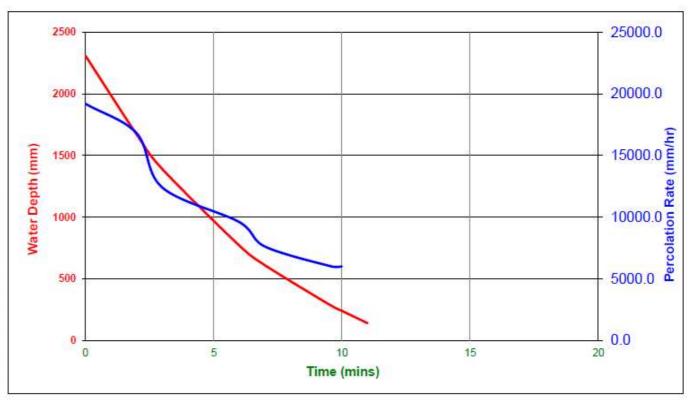
Average	2384.4	596.0941
Soakage Rate	(mm/hr)	
Soakage Rate	57.23	
per day	(m/day)	8

# Percolation Test Results (NZBC & Side and head wall)



Pipe Depth (m) = 2.50 m

e Depth (m) = Time	Water Level	Water Depth	Sookaga Area	Bore DIA (mm) = Soakaged Volume	Dercologation I
		water Depth	Soakage Area		
(minutes)	(mm below surface)	2242	(m <sup>2</sup> )	(m <sup>3</sup> )	(mm/hr)
2	190	2310	0.0050	0.00000	40000.0
2	830	1670	0.0050	0.00322	19200.0
3	1110	1390	0.0050	0.00141	16800.0
6	1730	770	0.0050	0.00312	12400.0
7	1890	610	0.0050	0.00080	9600.0
9.6	2220	280	0.0050	0.00166	7615.4
10	2260	240	0.0050	0.00020	6000.0
11	2360	140	0.0050	0.00050	6000.0
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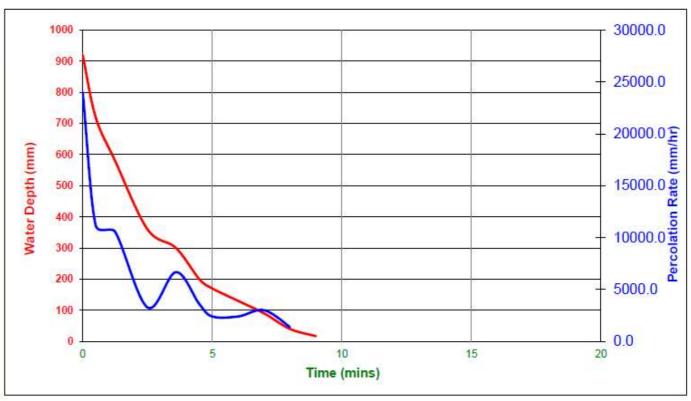


Average	11087.9	2771.978
Soakage Rate	(mm/hr)	1
Soakage Rate	266.11	+
per day	(m/day)	



Bore Depth (m) = 1.50 m Bore DIA (mm) = 80

e Depth (m) =	1.50 m			Bore DIA (mm) =	80
Time	Water Level	Water Depth	Soakage Area	Soakaged Volume	
(minutes)	(mm below surface)		(m <sup>2</sup> )	(m <sup>3</sup> )	(mm/hr)
0	580	920	0.0050	50 50 50 50 50 50 50 50 50 50 50 50 50 5	
0.5	780	720	0.0050	0.00101	24000.0
1.25	920	580	0.0050	0.00070	11200.0
2.5	1140	360	0.0050	0.00111	10560.0
3.6	1200	300	0.0050	0.00030	3272.7
4.5	1300	200	0.0050	0.00050	6666.7
5	1330	170	0.0050	0.00015	3600.0
6	1370	130	0.0050	0.00020	2400.0
7	1410	90	0.0050	0.00020	2400.0
8	1460	40	0.0050	0.00025	3000.0
9	1483	17	0.0050	0.00012	1380.0
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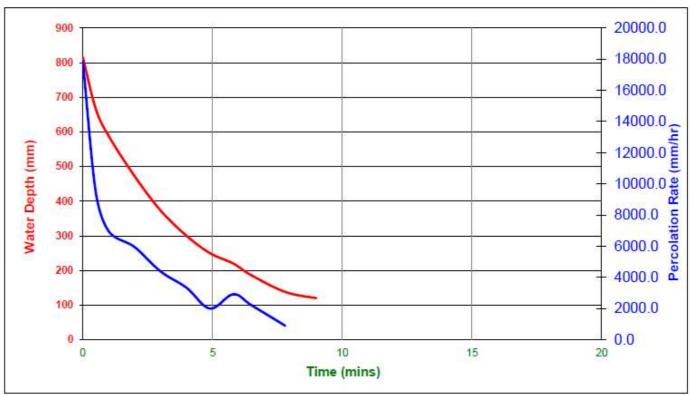
Average	6847.9	1711.985
Soakage Rate	(mm/hr)	8
Soakage Rate	164.35	
per day	(m/day)	1

# Percolation Test Results (NZBC & Side and head wall)



Location (NZGD Waikanae

Time	= 1.40 m Water Level	Water Depth	Soakage Area	Bore DIA (mm) = Soakaged Volume	Percolocation F
(minutes)	(mm below surface)		(m <sup>2</sup> )	(m <sup>3</sup> )	(mm/hr)
0	584	816	0.0050		(1111111)
0.5	733	667	0.0050	0.00075	17880.0
1	812	588	0.0050	0.00040	9480.0
2	928	472	0.0050	0.00058	6960.0
3	1027	373	0.0050	0.00050	5940.0
4	1100	300	0.0050	0.00037	4380.0
4.9	1150	250	0.0050	0.00025	3333.3
5.8	1180	220	0.0050	0.00015	2000.0
6.5	1214	186	0.0050	0.00017	2914.3
7.8	1262	138	0.0050	0.00024	2215.4
9	1280	120	0.0050	0.00009	900.0
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Average	5600.3	1400.075
Soakage Rate	(mm/hr)	
Soakage Rate	134.41	+
per day	(m/day)	



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