

# MEMO

**TO:** Eric Osborne

**DATE:** 20 July 2025

**FROM:** Tony Trueman

**PROJECT NO.:** J000814

**COPY:** Craig Martell, Susan Jones

**SUBJECT:** PEER REVIEW RESPONSES TO 100 & 110 TE MOANA ROAD, FLOOD ASSESSMENT & STORMWATER MANAGEMENT CONCEPT MEMO

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## INTRODUCTION:

Te Miro Water have made a further information request regarding the flood assessment & stormwater management concept memo supporting the plan change.

The following points have been raised by Te Miro Water.

1. *RFI – Update information provided around the effects of the Chillingworth stop bank breach.*
2. *RFI – Provide detailed soakage test information.*
3. *RFI – Provide full Geotechnical Report and method for establishing seasonal high groundwater elevation.*
4. *RFI – Provide a clear solution of how the stormwater associated risks are managed for flood plain filling and peak flow rate mitigation.*
5. *RFI – Provide a clear solution of how water quality and runoff volume will be managed.*

## Response – Point 1:

*RFI – Update information provided around the effects of the Chillingworth stop bank breach.*

There are impacts on peak depths within the site associated with the Chillingworth stop-bank breach scenario.

1. Breach flows move north-west toward the expressway following the lowest topography.
2. The low-lying ground adjacent the expressway stores some of the flow moving north.
3. Flow moves under the expressway and into the site.
4. Flow moves over Te Moana Road and into the Waimeha Stream

Peak depths within the site are in the order of 10 mm to 600 mm in this breach scenario.

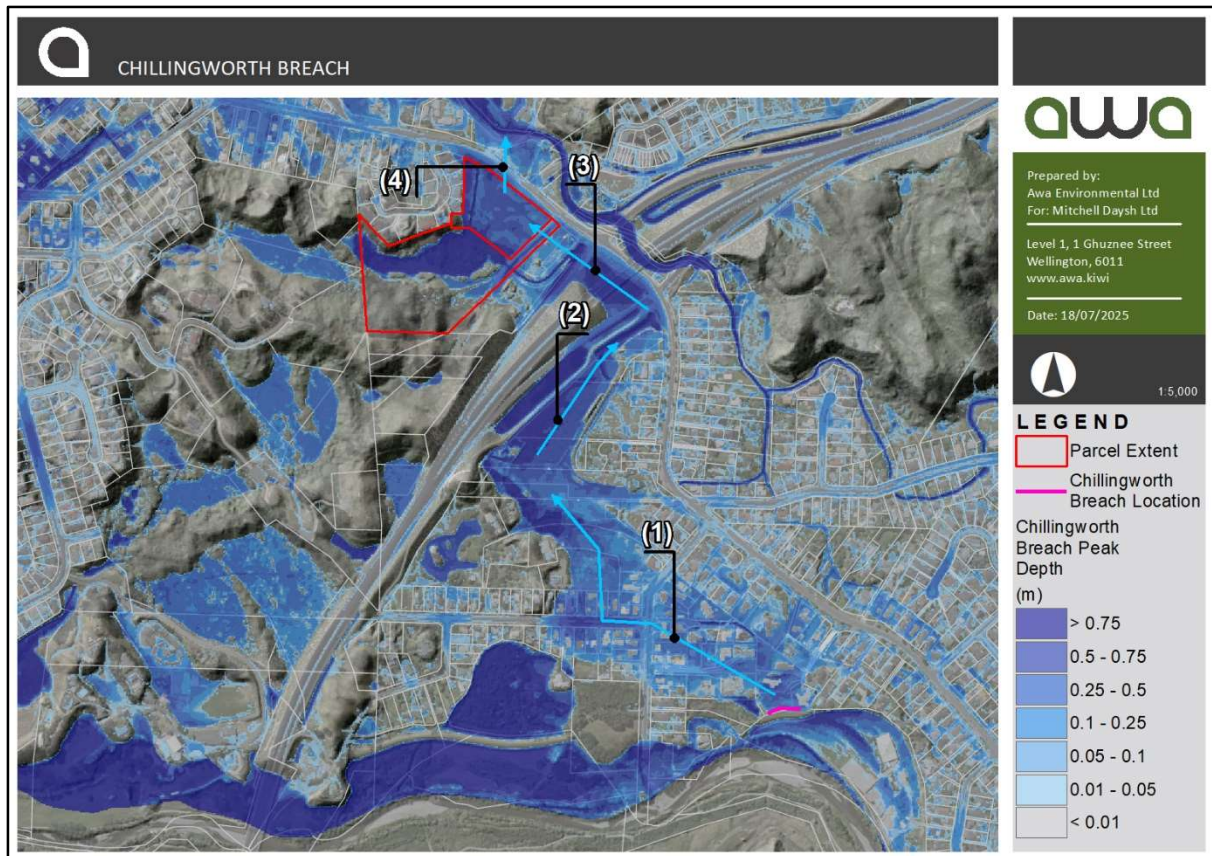


Figure 1 Chillingworth Stop-Bank Breach Scenario Peak Depths

There is sufficient scope, through site earthworks, to maintain the “effective functionally” of the residual overflow path through the site post development. A specific flood hazard assessment will be undertaken, including the development design, as part of future resource consent applications.

## Response – Point 2:

*RFI – Provide detailed soakage test information.*

See detailed soakage test information in Appendix 1.

## Response – Point 3:

*RFI – Provide full Geotechnical Report and method for establishing seasonal high groundwater elevation.*

The ground water levels have been shown to vary across the site in response to seasonal, wet weather events, soil type and local drainage. Awa and Cuttriss both encountered groundwater at varying depths using a hand auger. Within Lot 1 AWA encountered ground water at approximately 1.5 metres below ground level while Cuttriss encountered ground water at approximately 600mm below ground level. CGW have assumed a ground water 1.0 metre below ground level.

It is recommended by Cuttriss, CGW and AWA that standpipe piezometers are installed across the site along with regular water level monitoring to determine the groundwater level across the site.

## Response – Point 4:

*RFI – Provide a clear solution of how the stormwater associated risks are managed for flood plain filling and peak flow rate mitigation.*

This is a flood assessment & stormwater management concept memo for a plan change application. The purpose of the memo is to provide an overview of the local and regional flood hazard associated within the site and whether there is sufficient scope to mitigate development of the site. A specific flood hazard assessment will be undertaken, including the development design, as part of future resource consent applications.

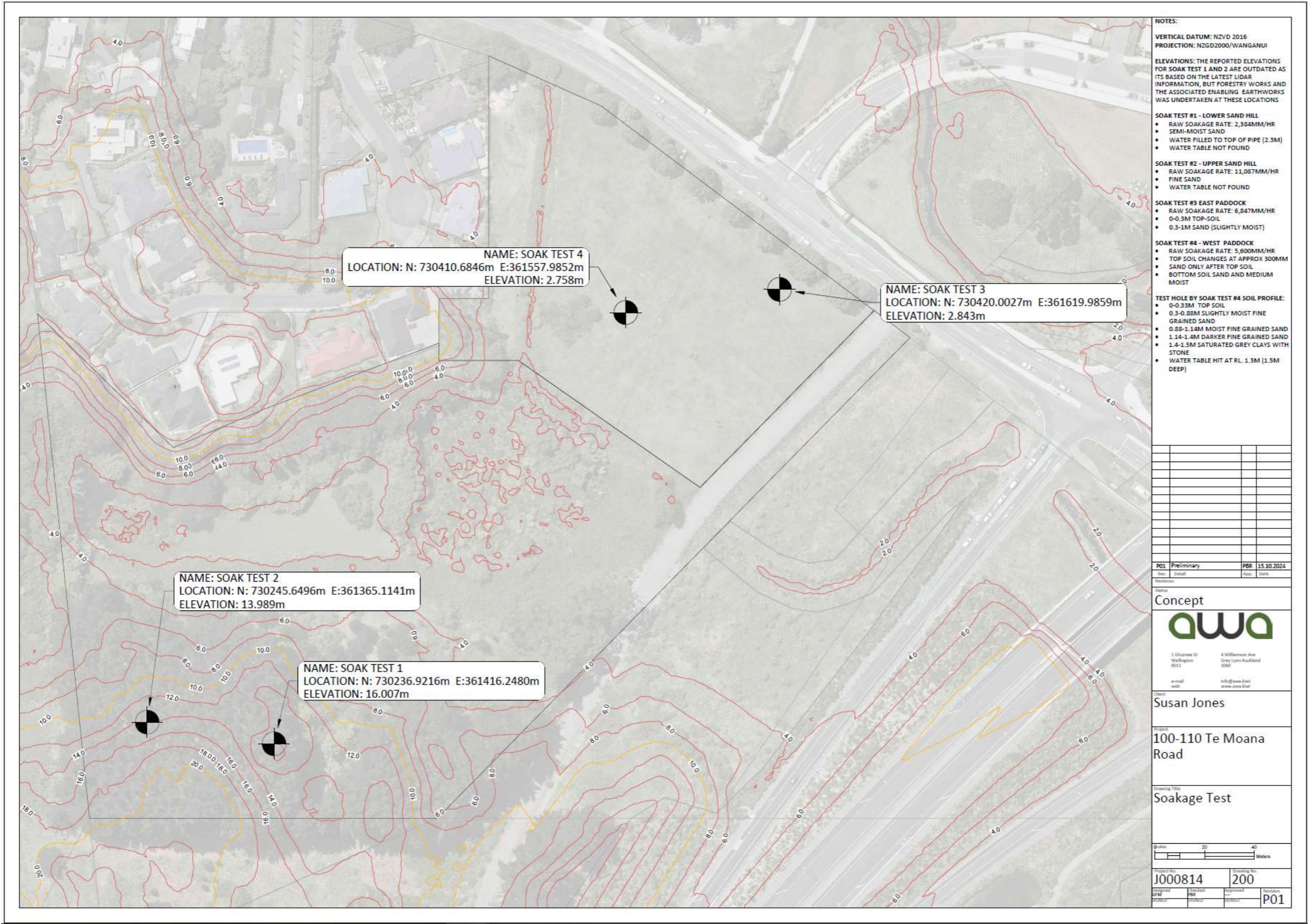
## Response – Point 5:

*RFI – Provide a clear solution of how water quality and runoff volume will be managed.*

There are several options available to maintain water quality across the site. These could take the form of infiltration or attenuation devices such as soakage/storage crates, bio-infiltration devices or constructed wetland areas. All these devices return runoff to ground in a diffuse manner which will mimic pre-development runoff patterns while maintain water quality.

A specific flood hazard assessment for mitigating runoff volume associated with the development design will be undertaken as part of future resource consent applications.

Appendix 1:



**Soak test 1**  
**Lower Sand hill**

**Percolation Test Results  
(NZBC & Side and head wall)**

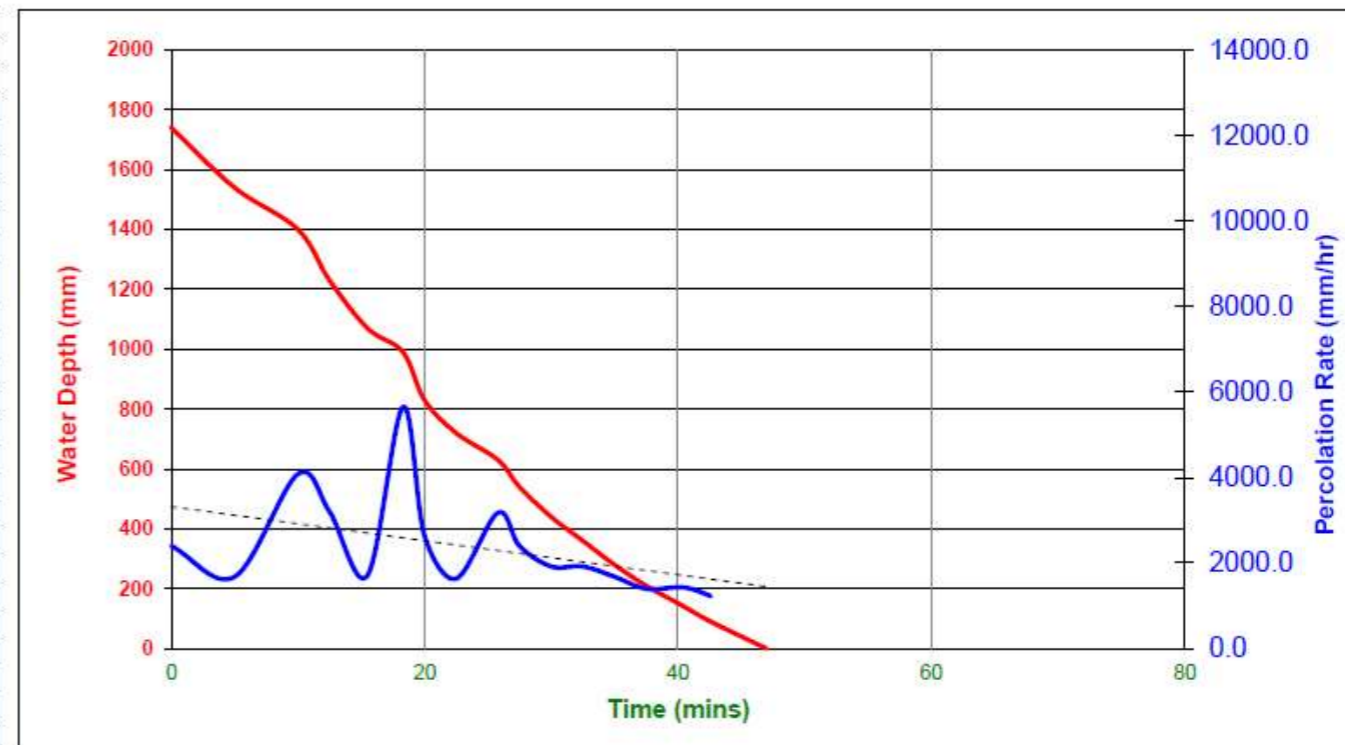


Pipe Depth (m) = 2.34 m

Bore DIA (mm) = 80

[illegible]

0.00875



Average	2384.4	596.0941
Soakage Rate	(mm/hr)	
Soakage Rate	57.23	
per day	(m/day)	

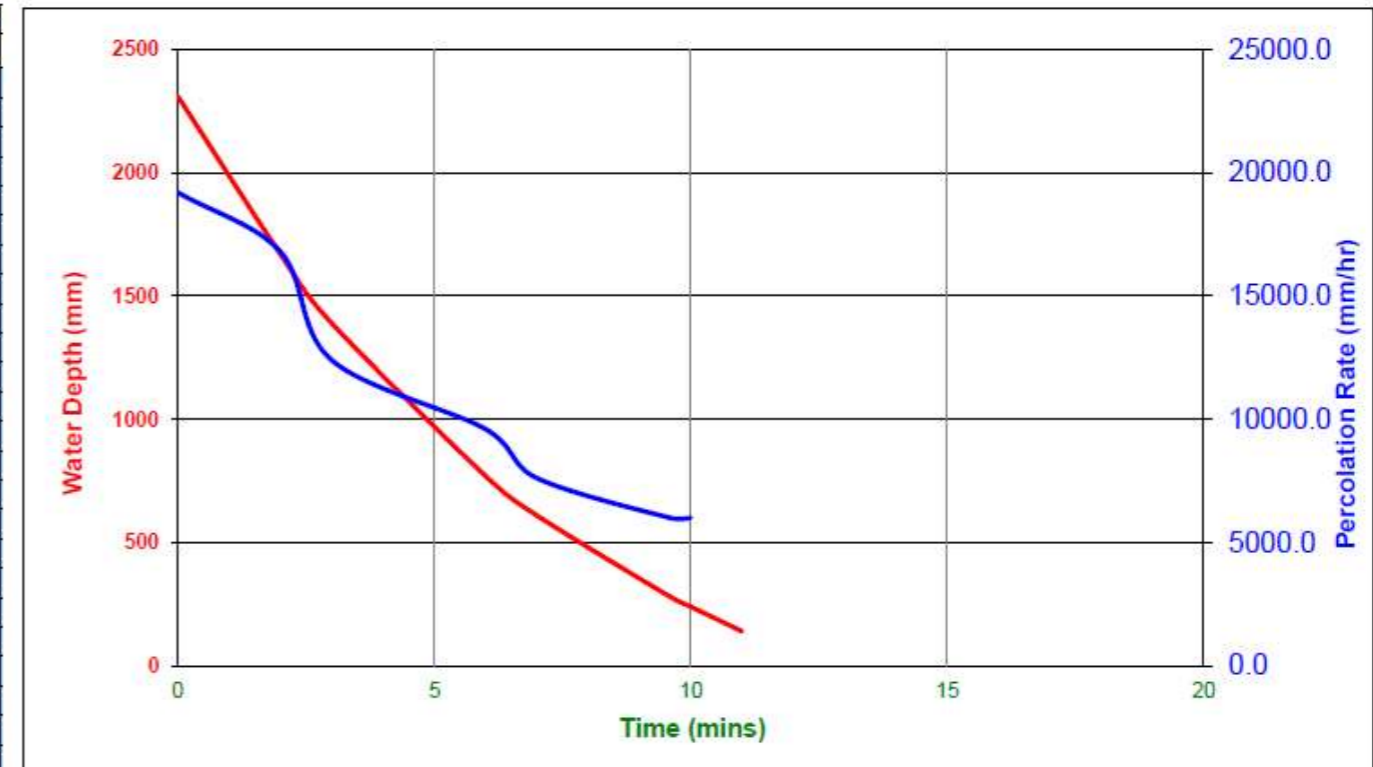
**Soak test 2**  
**Upper Sand Hill**

### Percolation Test Results (NZBC & Side and head wall)



Pipe Depth (m) = 2.50 m

Bore DIA (mm) = 80

[illegible]

Average	11087.9	2771.978
Soakage Rate	(mm/hr)	1
Soakage Rate	266.11	
per day	(m/day)	

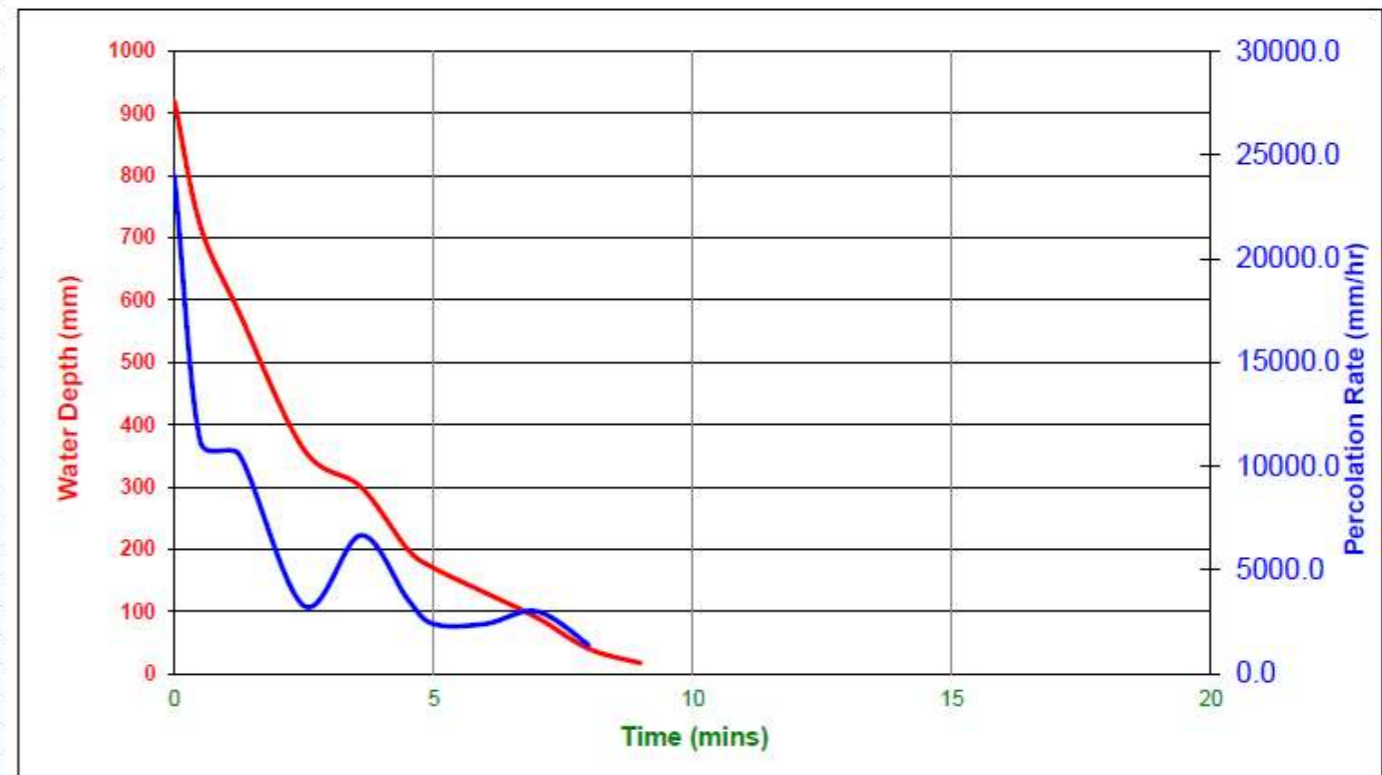
Soak test 3  
Left paddock

**Percolation Test Results  
(NZBC & Side and head wall)**



Bore Depth (m) = 1.50 m

Bore DIA (mm) = 80

[illegible]

Average	6847.9	1711.985
Soakage Rate	(mm/hr)	8
Soakage Rate	164.35	
per day	(m/day)	



Bore DIA (mm) = 80

The graph displays two variables over a 9-minute period. The red line, representing Water Depth, starts at 800 mm and decreases steadily to about 120 mm. The blue line, representing Percolation Rate, starts at 18000 mm/hr, drops sharply to around 2000 mm/hr within the first minute, and then fluctuates between 1000 and 2000 mm/hr for the rest of the time shown.

Time (mins)	Water Depth (mm)	Percolation Rate (mm/hr)
0	800	18000.0
1	600	2000.0
2	450	1500.0
3	350	1200.0
4	280	1500.0
5	220	1000.0
6	180	1500.0
7	150	1200.0
8	130	1000.0
9	120	1000.0

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